SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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5

STATE OF NORTH CAROLINA

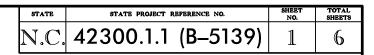
DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY PASQUOTANK

PROJECT DESCRIPTION BRIDGE NO. 690021 ON SR 1332 (CREEK ROAD) OVER KNOBBS CREEK

INVENTORY



CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6860. THE SUBSIFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALITORIED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONSTRUCTIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACTUAL CONDENSATIONS FOR ANY EXTENSION OF TIME FOR ANY RESON RESULTING FOR THE ACTUAL CONDITIONS TO BE COUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

- TES: THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. BY HAVING REDUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

DUGGINS, W. T.

FERGUSON, K. H.

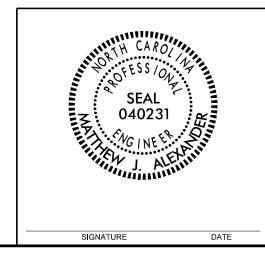
INVESTIGATED BY ______ CONSULTANTS

DRAWN BY <u>ALEXANDER</u>, M. J.

CHECKED BY ______ DENTON, R. L.

SUBMITTED BY ______ TERRACON CONSULTANTS

DATE JANUARY 2015



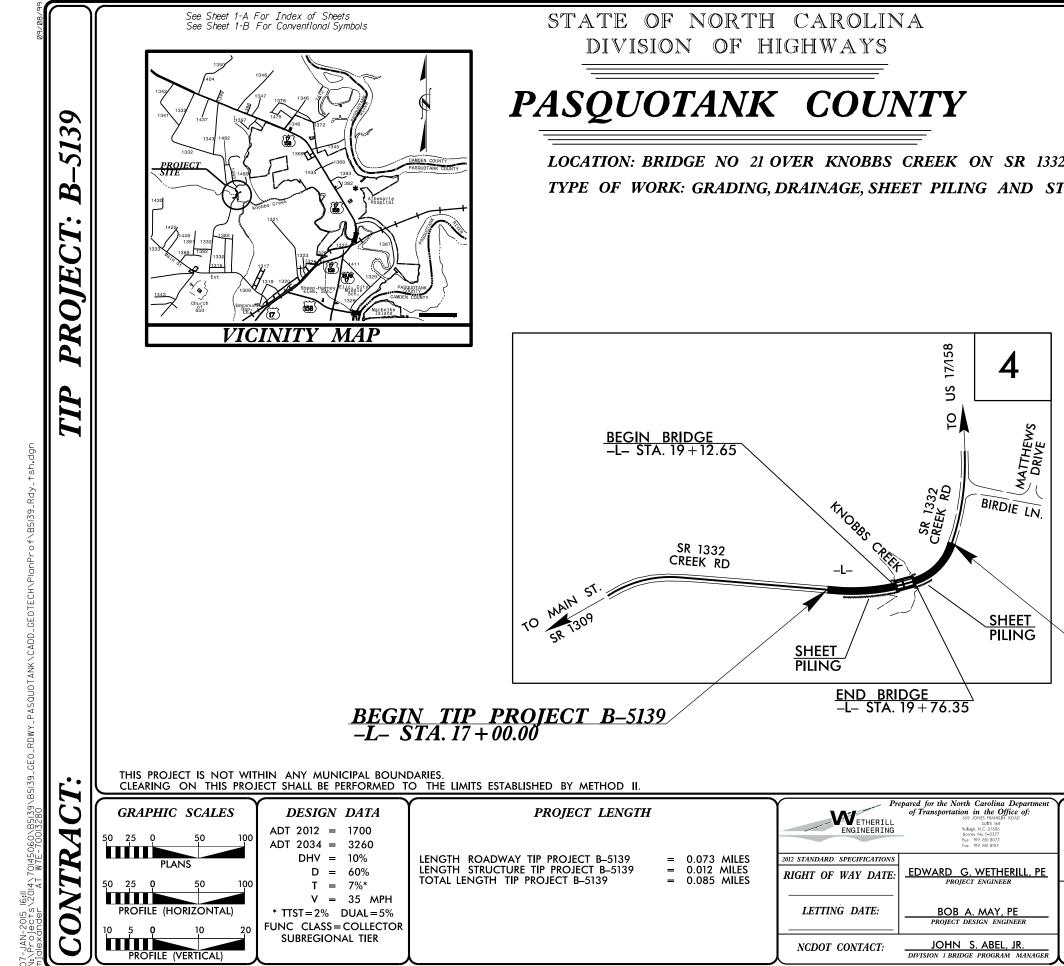
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

		CRADATION					TERMS AND DEFINITIONS
SOIL DESCRIPTION SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS T	AT CAN	GRADATION WELL GRADED - INDICATES A GOOD REPRESENTATION OF PA	RTICLE SIZES FROM FINE TO COARSE	HARD ROCK IS NON-COASTAL PLAIN MAT	ROCK DESCRIPTION	TESTED. AN INFERRED	
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS F ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586), SOIL CLASSIF	R FOOT	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE	ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT W	VHICH NON-COASTAL PLAIN MATERIAL WOULD PLIT SPOON SAMPLER EQUAL TO OR LESS T	YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOW	IG:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE		BLOWS IN NON-COASTAL PLAIN MATER	IAL, THE TRANSITION BETWEEN SOIL AND		ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTO AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLI	S SUCH	ANGULARITY OF GR		REPRESENTED BY A ZONE OF WEATHERE ROCK MATERIALS ARE TYPICALLY DIVID			ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6		THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS I ANGULAR, <u>SUBANGULAR, SUBROUNDED</u> , OR <u>ROUNDED</u> .	S DESIGNATED BY THE TERMS:	WEATHERED WING	-COASTAL PLAIN MATERIAL THAT WOULD YIE	LD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION		MINERALOGICAL COMP		ROCK (WR)	BLOWS PER FOOT IF TESTED.		ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) (> 35% PASSING *200)	ALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MIC		URISTALLINE UNIT NOU	TO COARSE GRAIN IGNEOUS AND METAMORF LD YIELD SPT REFUSAL IF TESTED. ROCK T		WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5		ARE USED IN DESCRIPTIONS WHEN THEY ARE CO		RUCK (LR)	ISS, GABBRO, SCHIST, ETC.		CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7		COMPRESSIBILIT	Y		E TO COARSE GRAIN METAMORPHIC AND NON- IMENTARY ROCK THAT WOULD YEILD SPT RE		COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL SCOOL		SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE	LL < 31		<u>K TYPE INCLUDES PHYLLITE, SLATE, SANDSTO STAL PLAIN SEDIMENTS CEMENTED INTO ROO</u>		OF SLOPE.
2 PASSING		HIGHLY COMPRESSIBLE	LL = 31 - 50 LL > 50	SEDIMENTARY ROCK	REFUSAL, ROCK TYPE INCLUDES LIMESTONE		CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY	MUCK,	PERCENTAGE OF MAT	ERIAL	(CP) SHEL	LL BEDS, ETC.		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 30 MX 50 MX 51 MN S01 X 51 MN S01 X 51 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 50 ILS S0ILS	PEAT	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS	OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRI	IGHT, FEW JOINTS MAY SHOW SLIGHT STAINING		ROCKS OR CUTS MASSIVE ROCK.
MATERIAL		TRACE OF ORGANIC MATTER 2 - 3% 3 - 5%	TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	IONT, FEW JOINTS PHT SHOW SEIGHT STHINING	. NOCK NINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING #40 SOILS WITH		LITTLE ORGANIC MATTER 3 - 5% 5 - 12% MODERATELY ORGANIC 5 - 10% 12 - 20%	LITTLE 10 - 20% SOME 20 - 35%		DINTS STAINED, SOME JOINTS MAY SHOW THIN		DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
LL – – – 40 MX 41 MN LITTLE OR PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN MODERATE	HIGHLY	HIGHLY ORGANIC > 10% > 20%	HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SF OF A CRYSTALLINE NATURE	PECIMEN FACE SHINE BRIGHTLY. ROCK RINGS L	NDER HAMMER BLOWS IF	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX Ø Ø Ø 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF	ORGANIC	GROUND WATEF	2		 DINTS STAINED AND DISCOLORATION EXTENDS	INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	SOILS	WATER LEVEL IN BORE HOLE IMM	EDIATELY AFTER DRILLING		CONTAIN CLAY, IN GRANITOID ROCKS SOME OCC		SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND SAND SAND GRAVEL AND SAND SOILS SOILS		STATIC WATER LEVEL AFTER 2	1 HOURS		DISCOLORED. CRYSTALLINE ROCKS RING UNDER ROCK SHOW DISCOLORATION AND WEATHERING		FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
CEN RATING				(MOD.) GRANITOID ROCKS, MOST FE	LDSPARS ARE DULL AND DISCOLORED, SOME SH	OW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR POOR	UNSUITABLE	0.000		DULL SOUND UNDER HAMME WITH FRESH ROCK.	R BLOWS AND SHOWS SIGNIFICANT LOSS OF S	RENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30		- ODUIL- SPRING OR SEEP			DISCOLORED OR STAINED. IN GRANITOID ROCK	S.ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS		MISCELLANEOUS SYN	1BOLS	SEVERE AND DISCOLORED AND A MA	AJORITY SHOW KAOLINIZATION. ROCK SHOWS SE	VERE LOSS OF STRENGTH	FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE		ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP	DIRECTION	(MOD. SEV.) AND CAN BE EXCAVATED WI IF TESTED, WOULD YIELD S	ITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SPT REFUSAL	SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
CONSISTENCY (N-VALUE) (TONS/F		WITH SOIL DESCRIPTION OF ROCK S			DISCOLORED OR STAINED. ROCK FABRIC CLEAF	AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4		SOIL SYMBOL	BORING SLOPE INDICATOR		STRONG SOIL. IN GRANITOID ROCKS ALL FELD RAGMENTS OF STRONG ROCK USUALLY REMAIN.	SPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LUUSE 4 10 10 MEDIUM DENSE 10 TO 30				IE TESTED, WOULD YIELD S			MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
(NON-COHESIVE) DENSE 30 TO 50		ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT AUGER BOF	ING CONE PENETROMETER	VERY ALL ROCK EXCEPT QUARTZ	DISCOLORED OR STAINED. ROCK FABRIC ELEM		USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERT DENSE 7 50			NG		REDUCED TO SOIL STATUS, WITH ONLY FRAGM AN EXAMPLE OF ROCK WEATHERED TO A DEGRE		PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VERY SOFT < 2 < 0.2 GENERALLY SOFT 2 TO 4 0.25 TO	0.5				CK FABRIC REMAIN. IF TESTED, WOULD YIELD		RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO MATERIAL STIFF 8 TO 15 1 TO		INFERRED ROCK LINE MONITORIN	G WELL - WITH CORE		OCK FABRIC NOT DISCERNIBLE.OR DISCERNIBLE NS. QUARTZ MAY BE PRESENT AS DIKES OR ST		ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF 15 TO 30 2 TO		ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY		ALSO AN EXAMPLE.	S. CONTIZ MAT DE FRESENT AS DIRES ON ST	NINGERS. SHEROLITE 15	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4					ROCK HARDNESS		SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE		RECOMMENDATION SY		VERY HARD CANNOT BE SCRATCHED BY	KNIFE OR SHARP PICK. BREAKING OF HAND SP	ECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 DPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053		UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE	ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF			SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
07ENING (MM) 4.76 2.00 0.42 0.23 0.075 0.055		SHALLOW UNCLASSIFIED EXCAVATION -	USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNI TO DETACH HAND SPECIMEN	IFE OR PICK ONLY WITH DIFFICULTY. HARD HA	MMER BLOWS REQUIRED	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BUULDER LUBBLE GRAVEL SAND SAND SILT	CLAY (CL.)				 IFE OR PICK. GOUGES OR GROOVES TO 0.25 IN	CHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
	NCE IN	ABBREVIATIONS			OF A GEOLOGIST'S PICK. HAND SPECIMENS CA	N BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.00 SIZE IN. 12 3		AR - AUGER REFUSAL MED MEDIUM BT - BORING TERMINATED MICA MICACEOUS	VST - VANE SHEAR TEST WEA WEATHERED	BY MODERATE BLOWS. MEDIUM CAN BE GROOVED OR GOUGE	ED 0.05 INCHES DEEP BY FIRM PRESSURE OF	KNIFE OR PICK POINT	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
		CL CLAY MOD MODERATELY	γ - unit weight	HARD CAN BE EXCAVATED IN SMA	ALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE B		WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS		CPT - CONE PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC	$\gamma_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S F		TER IN ERAON	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DE	CRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETE			D READILY BY KNIFE OR PICK. CAN BE EXCAVA INCHES IN SIZE BY MODERATE BLOWS OF A PI		STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USI		DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC e - VOID RATIO SD SAND, SANDY	S - BULK SS - SPLIT SPOON	PIECES CAN BE BROKEN BY	FINGER PRESSURE.		STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WAT		F - FINE SL SILT, SILTY	ST - SHELBY TUBE		E. CAN BE EXCAVATED READILY WITH POINT O IN BE BROKEN BY FINGER PRESSURE. CAN BE S		THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
		 FOSS FOSSILIFEROUS SLI SLIGHTLY FRAC FRACTURED, FRACTURES TCR - TRICONE REFUS 		FINGERNAIL			TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
BANGE - WET - (W) SEMISULIU; REQUIRES DRYING I		FRAGS FRAGMENTS W - MOISTURE CONTE	NT CBR - CALIFORNIA BEARING	FRACTURE SPACING	G BEDO)ING	BENCH MARK: BL-3 N:948063.72; E:281852.35
		HI HIGHLY V - VERY	RATIO	TERM SPACI		THICKNESS	
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM M	ISTURE	EQUIPMENT USED ON SUBJE		VERY WIDE MORE THAN WIDE 3 TO 10		4 FEET 1.5 - 4 FEET	ELEVATION: 3.26 FEET
SL _ SHRINKAGE LIMIT		DRILL UNITS: ADVANCING TOOLS:		MODERATELY CLOSE 1 TO 3		0.16 - 1.5 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER 1				CLOSE 0.16 TO VERY CLOSE LESS THAN	0.16 FEET THICKLY LAMINATED	0.03 - 0.16 FEET 0.008 - 0.03 FEET	
ATTAIN OPTIMUM MOISTURE		CME-55	CORE SIZE:			< 0.008 FEET	
PLASTICITY			В				
PLASTICITY INDEX (PI) DRY STREN		L CME-550 L HARD FACED FINGER BITS			S THE HARDENING OF MATERIAL BY CEMENT RUBBING WITH FINGER FREES NUMEROUS GR		
NON PLASTIC Ø-5 VERY LO SLIGHTLY PLASTIC 6-15 SLIGHT		VANE SHEAR TEST	HAND TOOLS:		GENTLE BLOW BY HAMMER DISINTEGRATES S		
MODERATELY PLASTIC 16-25 MEDIUM		X CASING W/ ADVANCER	POST HOLE DIGGER	MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE W	ITH STEEL PROBE:	
HIGHLY PLASTIC 26 OR MORE HIGH		PORTABLE HOIST X TRICONE 215/16 STEEL TE			BREAKS EASILY WHEN HIT WITH HAMMER.		
COLOR		X D-50 (TER255)	B. SOUNDING ROD		GRAINS ARE DIFFICULT TO SEPARATE WITH DIFFICULT TO BREAK WITH HAMMER.	STEEL PROBE:	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLU			VANE SHEAR TEST			CAMPI E.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANC	•		ЕТН		SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE BREAKS ACROSS GRAINS.	OHMITLE;	DATE: 8-15-14
							•

PROJECT REFERENCE NO. 42300.1.1 (B-5139)

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	STATE	STATI	S PROJECT REPERE		SHEET NO.	TOTAL SHEETS
	N.C.		<u>B–513</u>	9	3	6
		20011	P.A.PRO	J. NQ.	DESCRIPT	ION
		300.1.1 300.2.1				UTIL.
		300.3.1			CON	
A (CDEEV DI	<u></u>		1	L		
B2 (CREEK RL	J .)					
TRUCTURE	8320	J.				
ENI -L-) <u>1</u> ST2	<u>TIP 1</u> 4. 21 -	PRE	ECT 0	Y PLA	NS
HYDRAULICS EN	IGINEEI	R	$\overline{\gamma}$			=
SIGNATURE: ROADWAY DE ENGINEEK	<u>P.E.</u> SIGN	<u>.</u>	(OF NOI	A CINELING CONTRACTOR	

lerracon

Wetherill Engineering, Inc. 559 Jones Franklin Road, Suite 164 Raleigh, North Carolina 27606

Mr. Edward G. Wetherill, PE Attn:

Re: **Geotechnical Report - Roadway Inventory**

Bridge No. 690021 on SR 1332 (Creek Road) over Knobbs Creek Pasquotank County, North Carolina Project No. 42300.1.1 TIP No. B-5139 Terracon Project No. 70145060

Project Description

This project consists of the replacement of an existing 0.074 miles of two lane roadway and a 0.011 mile structure on SR 1332 (Creek Road) between Pot O Gold Trail and Birdie Lane. The project includes the construction of a permanent sheet pile wall at both end bents of the proposed structure.

The subsurface investigation was performed during May of 2014. A track-mounted Diedrich D-50T drill machine equipped with an automatic hammer was used during the investigation. Standard Penetration Tests were performed in the boreholes and samples were collected for visual classification in the field.

The following alignments were investigated. Subsurface profiles and selected cross sections of these alignments are included in this report.

Physiography and Geology

The project is located in the floodplain of Knobbs Creek. The topography is flat and the drainage from the area is very poor.

The project is located within the Outer Coastal Plain Physiographic Province. More specifically, the project geology is composed of undivided surficial deposits. The soils along the project were deposited during periods of fluctuating shoreline.

Soil Properties

Soils within the project area include roadway embankment fill and coastal plain deposits.

The roadway embankment fill was encountered along the existing roadway and bridge approaches. This fill material consists of brown, wet, clayey sand (A-2-6).

A layer of alluvial muck and peat was encountered below the roadway embankment fill in each of the borings performed along the project.

The coastal plain soils encountered along the project consist of alternating layers of sandy clay (A-6), clay (A-7), silty and clayey sand (A-2-4 and A-2-6, respectively), and sand (A-3). The cohesive soils at the site were generally gray in color and very soft to soft. The sands at the site were also gray in color and were loose to dense.

Groundwater

Groundwater was encountered within approximately 3 feet of existing grades in the borings performed at the site.

Line	<u>Station</u>	Respectfully Submitted,
		Terracon Consultants, Inc.
-L-	17+00 to 21+50	
Areas of Spec	ial Geotechnical Interest	

1) Ground water was encountered within approximately 3 feet of existing grades at the following locations:

Matthew J. Alexander, PE **Geotechnical Project Engineer**

<u>Line</u>	<u>Station</u>
-L-	17+00 to 21+50

2) Highly organic soils consisting of muck and peat were encountered at the following locations:

	Line	<u>Station</u>	
	-L-	17+00 to 21+50	
3	Terracon Consultants, Inc. 240 P [919] 873 2211 F [919] 873 9555 ter	1 Brentwood Road Raleigh, NC 27604 racon.com North Carolina Registered F-0869	9
	Geotechnical 🗖 Environmental	Construction Materials	Facilitie

Project Reference No.	Sheet No.
42300.1.1 (B-5139)	3A

R. L. "Levi" Denton, II, PE Principal, Geotechnical Department Manager

